Modeling of shell–beam transitions in the presence of finite rotations

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A finite element formulation for a transition element between shells and beam structures is described in this paper. The elements should allow changes between models in an ‘optimal’ way without or with little disturbances which decrease rapidly due to the principle of Saint-Venant. Thus, the constraints are formulated in such a way that a transverse contraction within the coupling range is possible. The implementation of the coupling conditions is done with the Penalty Method or the Augmented Lagrange Method. The element formulation is derived for finite rotations. Same rotational formulations are used in beam and shell elements. Rotational increments up to an angle of $2\pi$ are possible without singularities based on a multiplicative update procedure. It can be shown that the transition to rigid bodies can be derived with some modifications. Examples prove the reliability of the transition formulation. Here simple element tests and practical applications are shown.

1. INTRODUCTION

The process of detailed modeling of thin-walled beam structures may lead to large systems and a complex numerical analysis. Even with modern computational equipment this task should be managed with problem oriented techniques. Parts of thin 3D-structures where only global behaviour is of interest can be discretized with beam elements whereas regions where for example local stability phenomena occur can be modeled with shell elements. Thus, transition elements have to be developed, which should combine both types of elements in a proper way. In general these elements are available in most of the commercial finite element codes. But simple examples show that they may lead to severe disturbances of the local stress state. This is especially important when considering material nonlinear behaviour.

2. FORMULATION OF CONSTRAINT EQUATIONS

The aim of this paper is to develop finite elements which describe the transition between shells and 3D-beams in such a way that the constraints imposed on both formulations are minimal. The description should hold for any case of geometrical nonlinearities. Therefore the elements have to be able to describe finite rotations. As basic kinematic assumption for the element we assume

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Fig. 1. Example: cross section of an U-shaped beam with shell and beam discretization

Fig. 2. Reference and current configuration of the cross section at transition

- an undeformable plane cross section in accordance with the beam theory. No warping effects are allowed (this may be defined for the beam element by default whereas it has to be enforced for the shell element as constraint).

- All nodes of the shell discretization which have to be coupled as well as the beam node are situated in this plane, see Fig. 1.

- All nodes in the cross section can move on a straight line through the center of gravity. (In a beam theory with elastic material behaviour we assume $\sigma_y = \sigma_z \approx 0$, which leads to $\varepsilon_x = \frac{P}{EI}$, $\varepsilon_y = -\nu \varepsilon_x$, $\varepsilon_z = -\nu \varepsilon_x$. Thus, an arbitrary point of the cross section can move only on a straight line through the center of gravity.)

Points on the cross section can be described by a convective coordinate system $\xi_2, \xi_3$ with base vectors $\mathbf{A}_2, \mathbf{A}_3$ in the undeformed and $\mathbf{a}_2, \mathbf{a}_3$ in the deformed configuration, see Fig. 2. Here the orthonormal base systems,

$$\mathbf{A}_i = R_0 \mathbf{e}_i = (A_i \otimes \mathbf{e}_i) \mathbf{e}_i,$$

$$\mathbf{a}_i = R \mathbf{e}_i = (a_i \otimes \mathbf{e}_i) \mathbf{e}_i = \Delta R R_0 \mathbf{e}_i,$$  \hspace{1cm} (1)
are introduced. Beam elements based on Bernoulli theory or Timoshenko theory can be used.

In the undeformed configuration an arbitrary point \( I \) of the cross section is defined by

\[
X_I = X_0 + \xi_2 A_2 + \xi_3 A_3.
\]  

(2)

The coordinates of this point \( I (0, \xi_2, \xi_3) \) on the cross section can be calculated using the base vectors \( A_1 \),

\[
\xi_2 = (X_I - X_0) \cdot A_2, \quad \xi_3 = (X_I - X_0) \cdot A_3.
\]  

(3)

An arbitrary point \( I \) of the cross section in the current configuration is found if

\[
x_I = x_0 + c_2 a_2 + c_3 a_3, \quad c_2, c_3 \in \mathcal{R}
\]  

(4)

holds.

Due to the fact that points move on a straight line through the center of gravity \( S \) during the deformation the position of this point can be described using a parameter \( \lambda \)

\[
x_I = x_0 + \lambda (\xi_2 a_2 + \xi_3 a_3), \quad \lambda \in \mathcal{R}.
\]  

(5)

Here, \( \lambda \) can be defined from the distance between \( S \) and \( I \) in the undeformed and current configuration,

\[
\lambda = \frac{\|x_I - x_0\|}{\|X_I - X_0\|}.
\]  

(6)

Thus, the following constraint equation holds for each point of the shell cross section

\[
f = x_I - x_0 - \lambda (\xi_2 a_2 + \xi_3 a_3) = 0.
\]  

(7)

Based on the above defined assumptions warping cannot be described. This may be crucial in some situations.

3. Finite Element Formulation

Based on the above described kinematical assumptions the element is developed. The beam node in the transition cross-section is called 'reference node'. Furthermore, the base vectors \( A_2 \) and \( A_3 \) define the orientation of the cross section. It is assumed that the shell nodes to couple ('coupling nodes') lie in this plane. The vectors \( A_2 \) and \( A_3 \) are used to specify the section coordinates, see Eq. (3). In the current configuration the base vectors \( a_2 \) and \( a_3 \) of the beam element together with the convective coordinates \( (0, \xi_2, \xi_3) \) and the parameter \( \lambda \) define the coupling nodes.

The mechanical model of the cross section can be considered as a sum of rigid beams which allow only for axial deflections. The boundary conditions are clamped at the reference node and jointed at the coupling node, see Fig. 3.

The implementation of the constraint equation (7) in a transition element is done via the Penalty and the Augmented Lagrange Method. Furthermore a consistent linearization is derived for the element with respect to finite rotations. The transition is formulated between an arbitrary shell node and the associated beam node as a two node element. Thus Fig. 3 may be interpreted as the system of all necessary transition elements for the shown case. The unknown values at both nodes of the element are introduced in the following vector,

\[
\mathbf{v}^{(e)} = \begin{bmatrix} u_I \\ \omega_I \\ u_0 \\ \omega_0 \end{bmatrix}^{(e)},
\]  

(8)
where \( u \) are the displacements and \( \omega \) are the rotational quantities. The index '0' refers to the beam node whereas the index 'I' is used for the coupled shell node.

The equations of the penalty method, see e.g. [1, 6], are summarized in the following,

\[
\Pi_P = \Pi(u) + \Pi = \Pi(u) + \frac{1}{2} \alpha f^T f, \quad \alpha \in \mathcal{R},
\]

\[
\delta \Pi_P = \delta \Pi(u) + \delta \Pi = \delta \Pi(u) + \alpha \delta f^T f,
\]

\[
\Delta \delta \Pi_P = \Delta \delta \Pi(u) + \Delta \delta \Pi = \Delta \delta \Pi(u) + \alpha \delta f^T \Delta f + \alpha \Delta \delta f^T f.
\]

A more stable method is introduced by the Augmented Lagrange Method, see e.g. [4, 7, 8], with the following equations.

\[
\Pi_A = \Pi(u) + \Pi = \Pi(u) + \frac{1}{2} \alpha f^T f + \Lambda^T f,
\]

\[
\delta \Pi_A = \delta \Pi(u) + \delta \Pi = \delta \Pi(u) + \delta f^T (\alpha f + \Lambda),
\]

\[
\Delta \delta \Pi_A = \Delta \delta \Pi(u) + \Delta \delta \Pi = \Delta \delta \Pi(u) + \alpha \delta f^T \Delta f + \Delta \delta (\alpha f + \Lambda).
\]

Here, \( \alpha \) is the Penalty parameter which has to be chosen and \( \Lambda \) are the Lagrangian multipliers. These are held constant within an iteration step. The update reads

\[
\Lambda_{i+1} = \alpha f_i + \Lambda_i \quad \text{with} \quad \Lambda_0 = 0.
\]

To derive the necessary equations for both methods simultaneously we introduce the vector \( \tilde{\Lambda} \) with

\[
\tilde{\Lambda} = \alpha f \quad \text{(Penalty)}, \quad \tilde{\Lambda} = \alpha f + \Lambda \quad \text{(Augmented Lagrange)}.
\]

To determine the residual and the stiffness matrix we need to specify the variation and linearization of the constraints. If we denote the position vector from the reference node to the coupling node with respect to the basis \( a_i \) with

\[
r_I = \xi_2 a_2 + \xi_3 a_3,
\]

it holds for

\[
f = x_I - x_0 - \lambda r_I
\]

and its variation

\[
\delta f = \delta u_I - \delta u_0 - \delta \lambda r_I - \lambda \delta r_I.
\]
Here $\delta \lambda$ and $\delta r_I$ have to be derived. We introduce the distance $R$ of the nodes in the reference state

$$ R = \|X_I - X_0\|. \quad (16) $$

In the current configuration the distance of these nodes is defined by

$$ \|X_I - X_0\| = \sqrt{(x_I - x_0) \cdot (x_I - x_0)}. \quad (17) $$

Starting from Eq. (6) and using the chain rule and product rule it holds for $\delta \lambda$

$$ \delta \lambda = \frac{1}{R} \left( \frac{\delta u_I - \delta u_0}{2 \sqrt{(x_I - x_0) \cdot (x_I - x_0)}} \cdot (x_I - x_0) + (x_I - x_0) \cdot (\delta u_I - \delta u_0) \right) \quad (18) $$

With the abbreviations

$$ n = \frac{x_I - x_0}{\|x_I - x_0\|} \quad \text{and} \quad \hat{n} = \frac{1}{R} n, \quad (19) $$

$\delta \lambda$ can be written finally as

$$ \delta \lambda = \hat{n} \cdot (\delta u_I - \delta u_0). \quad (20) $$

The second term to be derived is the variation $\delta r_I$. With

$$ \delta r_I = \xi_2 \delta a_2 + \xi_3 \delta a_3 \quad (21) $$

and

$$ \delta a_i = \delta \omega_0 \times a_i, \quad (22) $$

$\delta r_I$ can be stated as

$$ \delta r_I = \xi_2 \delta \omega_0 \times a_2 + \xi_3 \delta \omega_0 \times a_3. \quad (23) $$

Thus, $\delta r_I$ is defined in terms of $r_I$ and $\delta \omega_0$

$$ \delta r_I = \delta \omega_0 \times r_I = W \delta \omega_0 \quad (24) $$

with

$$ W = -\text{skew} r_I = \begin{bmatrix} 0 & r_{3I} & -r_{2I} \\ -r_{3I} & 0 & r_{1I} \\ r_{2I} & -r_{1I} & 0 \end{bmatrix}. \quad (25) $$

The axial vector $\omega_0$ contains the rotational degrees of freedom of the reference node.

Finally the variation of the constraint can be written as

$$ \delta f = \delta u_I - \delta u_0 - [\hat{n} \cdot (\delta u_I - \delta u_0)] r_I - \lambda W \delta \omega_0 $$

$$ = (1 - r_I \otimes \hat{n})(\delta u_I - \delta u_0) - \lambda W \delta \omega_0 $$

$$ = A(\delta u_I - \delta u_0) - \lambda W \delta \omega_0 \quad (26) $$

with

$$ A = 1 - r_I \otimes \hat{n}. \quad (27) $$
Thus, the residual for a single transition element is defined by

$$\delta \Pi^{(e)} = \delta \nu^{T(e)} G^{(e)} = \delta \nu^{T(e)} B_f^T \tilde{\Lambda} = [\delta u_f^T, \delta \omega_f^T, \delta u_0^T, \delta \omega_0^T]^{(e)} \begin{bmatrix} A^T & 0 & -A^T & -\lambda W^T \\ 0 & -A^T \\ -\lambda W^T \\ -\lambda W^T \end{bmatrix} \tilde{\Lambda}.$$  (28)

The necessary terms in Eqs. (9), (10) are derived in a similar way and the tangent stiffness matrix follows from

$$k^{(e)}_T = B_f^T \alpha B_f + \begin{bmatrix} \tilde{P} & 0 & -\tilde{P} & -F^T \\ 0 & 0 & 0 & 0 \\ -\tilde{P} & 0 & \tilde{P} & F^T \\ -F & 0 & F & H \end{bmatrix}$$  (29)

with

$$\tilde{P} = - (\tilde{\Lambda} \cdot r_f) \tilde{P},$$
$$\tilde{P} = \frac{1}{R} \frac{1}{\|x_f - x_0\|} \left[ 1 - (n \otimes n) \right],$$
$$F = (r_f \times \tilde{\Lambda}) \otimes \tilde{n},$$
$$H = -\lambda \left[ \frac{1}{2} (r_f \otimes \tilde{\Lambda} + \tilde{\Lambda} \otimes r_f) - (\tilde{\Lambda} \cdot r_f) I \right].$$  (30)

4. RIGID BODY TRANSITION ELEMENT

In the following we discuss the application of the above derived equations on the transition between rigid and flexible parts of structures. The deformation of a rigid body is described by the translation and the rotation of a reference point 'o'. Any other points on the rigid body can be described by a convective coordinate system $\xi_1, \xi_2, \xi_3$ with base vectors $A_1, A_2, A_3$ in the undeformed and $a_1, a_2, a_3$ in the deformed configuration, see Eqs. (2) and (3). Furthermore the same orthonormal base systems $A_i$ and $a_i$ (Eq. (1)) are used. These other points may be points were a transition to flexible parts (for example a shell) of the structure occur. It holds for the current configuration, see Eqs. (5), (13)

$$x_f = x_0 + (\xi_1 a_1 + \xi_2 a_2 + \xi_3 a_3) = x_0 + r_f$$  (31)

and the constraint is defined by, see Eq. (7)

$$f = x_f - x_0 - (\xi_1 a_1 + \xi_2 a_2 + \xi_3 a_3) = (X_f + u_f) - (X_0 + u_0) - r_f = 0.$$  (32)

Thus, the equations derived in Sections 2 and 3 can be used with $\lambda = 1$.

With $\delta r_f = \delta \omega_0 \times r_f = W \delta \omega_0$, see Eqs. (24)–(25), the variation of the constraint yields

$$\delta f = \delta u_f - \delta u_0 - \delta r_f = \delta u_f - \delta u_0 - W \delta \omega_0.$$  (33)

Finally the residual for a single transition element is

$$G^{(e)} = B_f^T \tilde{\Lambda} = \begin{bmatrix} 1 \\ 0 \\ -1 \\ -W^T \end{bmatrix} \tilde{\Lambda}.$$  (34)
The mechanical model of the transition between rigid and flexible parts is now interpretable as a sum of pure rigid beam elements with clamped boundary conditions at the reference node and jointed boundary conditions at the coupling nodes, see Fig. 4.

Next we derive the necessary linearization for the transition element. Similar to $\delta f$ the linearization $\Delta f$ can be written as

$$\Delta f = B_f \Delta v^{(e)} = \begin{bmatrix} \Delta u_I \\ \Delta \omega_I \\ \Delta u_0 \\ \Delta \omega_0 \end{bmatrix}.$$  

Furthermore the term $\Delta \delta f$ has to be derived:

$$\Delta \delta f = -\Delta \delta r_I = - (\xi_1 \Delta \nu a_1 + \xi_2 \Delta \nu a_2 + \xi_3 \Delta \nu a_3)$$

with

$$\Delta \nu a_i = \nu \Delta \nu a_i \times \nu a_i = \nu \Delta \nu a_i \times (\nu a_i \times \nu a_i)$$

$$= (\nu a_i \cdot \nu a_i) \Delta \nu a_i - (\nu a_i \cdot \Delta \nu a_i) a_i = (\nu a_i \otimes \Delta \nu a_i) a_i - (\nu a_i \cdot \nu a_i) a_i,$$

which finally leads to

$$\Delta \delta f = -[(\nu a_i \otimes \Delta \nu a_i) - (\nu a_i \cdot \nu a_i) 1] r_I.$$

Thus, the term $\Delta \delta f^T (\alpha f + \Lambda)$ for the linearization is given by

$$\Delta \delta f^T (\alpha f + \Lambda) = - (\alpha f + \Lambda)^T [\nu a_i \otimes \Delta \nu a_i - (\nu a_i \cdot \nu a_i) 1] r_I$$

$$= - (\alpha f + \Lambda)^T [(\nu a_i \cdot r_I) \Delta \nu a_i - (\nu a_i \cdot \nu a_i) r_I]$$

$$= \nu a_i^T (- (\alpha f + \Lambda) r_I^T + (\alpha f + \Lambda)^T r_I 1) \Delta \nu a_i$$

$$= \nu a_i^T \alpha H \Delta \nu a_i$$

and we end up with the associated tangent stiffness matrix on element level

$$k^{(e)}_T = \alpha \begin{bmatrix} 1 & 0 & 1 & -W \\ 0 & 0 & 0 & 0 \\ -1 & 0 & 1 & W \\ -W^T & 0 & W^T & W + H \end{bmatrix}.$$
with

\[
H = \left( f + \frac{1}{\alpha} A \right)^T r_i \mathbf{1} - \left( f + \frac{1}{\alpha} A \right) r_i^T \\
\approx \left( f + \frac{1}{\alpha} A \right)^T r_i \mathbf{1} - \frac{1}{2} \left( \left( f + \frac{1}{\alpha} A \right) r_i^T + r_i \left( f + \frac{1}{\alpha} A \right)^T \right).
\]

(41)

Now all equations for the implementation in a finite element code are derived.

5. EX"MPLES

The developed finite element formulations for both transition elements have been implemented in an enhanced version of the program FEAP, documented in a basic version in [10].

5.1. Clamped thin-walled H-beam

The first example demonstrates the effect of a rigid and soft transition element for the case of a clamped thin-walled H-beam under axial load. The first part of the beam is modeled with beam elements [2, 9], the second part is discretized using shell elements [3]. Geometry and material data are shown in Fig. 5. In Fig. 6 the transverse normal stresses are plotted for the shell region. It can be seen clearly that in the rigid case stresses occur due to obstruction of the transverse deformations. The magnitude of these stresses \( \sigma_{22} \) amounts to more than 20 \% of the longitudinal stress \( \sigma_{11} \), see Fig. 7.

The distribution of normal and transverse stresses along a line through the center of gravity is depicted in Fig. 7. It can be seen clearly that there is no disturbance of the stress state if the soft transition elements are used.

\[ F = 2000 \text{ kN} \]
\[ A = 80 \text{ cm}^2 \]
\[ E = 21000 \text{ kN/cm}^2 \]
\[ \nu = 0.3 \]

Fig. 5. Clamped beam under axial load
Fig. 6. Transverse normal stresses for soft and rigid transition element

Fig. 7. Distribution of stresses along a line (coordinate x) through the center of gravity
5.2. Steel frame structure

The second example demonstrates coupling of beam and shell elements for a steel frame structure with welded cross sections, see Fig. 8. The different cross sections are depicted in Fig. 9. The frame is fixed in z-direction at \( x = (-6, -3, 0, 3, 6) \) m, \( y = 4 \) m. Elastic material behaviour is assumed using the parameter \( E = 21000 \text{ kN/cm}^2 \) and \( \nu = 0.3 \).

The dimensions of the haunch are shown in Fig. 10. It is assumed that leg and member are coupled with rigid welding seams.

For comparison we model the structure in 3 different ways, see Fig. 11. The position of the transitions are defined in regions where the beam theory holds and no disturbances of the stress state may occur. Here the transitions are at \( x = 4.14 \) m in the member and at \( y = 2.60 \) m in the leg.

Results of the nonlinear analysis for the vertical deflection at \( x = 0 \) are depicted in Table 1 for the different discretization models. Deviations are calculated under the assumption that the pure shell solution leads to the best results. Only slightly differences occur between the different models. Due to the simplifications in the pure beam model (haunch) no comments can be given on the fact that the pure beam model is slightly more accurate then the mixed model.

In Figs. 12 and 13 the longitudinal stresses are depicted for the beam–shell and the pure shell model. Again nearly no differences can be seen between the different discretization strategies. The maximum differences in the stresses \( \sigma_{11} \) are about 4% in the transition zone. Thus, the derived transition elements can be used efficiently in coupling these types of elements without nearly any disturbances of stress and deformation state.

![Frame structure: system and loading](image)

**Fig. 8. Frame structure: system and loading**

![Definition of cross sections](image)

**Fig. 9. Definition of cross sections**
Beam model
Here, 20 beam elements are used. Within the haunch 5 elements with constant cross sections are used.

Beam–shell model
For the shell part we use 720 shell elements whereas the beam part is modeled with 28 beam elements. At each transition 16 transition elements are used.

Shell model
The whole system is modeled with 1744 shell elements.
Table 1. Vertical displacements at center of horizontal member

<table>
<thead>
<tr>
<th>Model</th>
<th>displacement</th>
<th>deviation</th>
</tr>
</thead>
<tbody>
<tr>
<td>beam</td>
<td>1.8634 cm</td>
<td>-2.06 %</td>
</tr>
<tr>
<td>beam–shell</td>
<td>1.8580 cm</td>
<td>-2.34 %</td>
</tr>
<tr>
<td>shell</td>
<td>1.9025 cm</td>
<td>–</td>
</tr>
</tbody>
</table>

Fig. 12. Normal stresses, beam–shell model

Fig. 13. Normal stresses, shell model
5.3. Clamped beam under end moment

In the third example we discuss the modeling of a clamped beam under end moments using volume elements, e.g., [5]. Here, the question arises how to model the application of the end moment. A simple method is to model the beam tip with rigid transition elements. System and loading are shown in Fig. 14 whereas different deformed meshes and the material parameter are depicted in Fig. 15. No difficulties arise and the problem can be calculated using 5 incremental steps until a moment of $M = 2\pi EI/\ell$ which leads to a complete circle of the deformed mesh. Thus the transition elements describe the application of the moment on the volume elements correctly.

![Fig. 14. Clamped beam under end moment](image)

![Fig. 15. Deformed meshes for clamped beam under end moment](image)

6. CONCLUSIONS

A finite element formulation for a transition element between shells and beam structures has been described. The coupling condition is formulated such that transverse contraction within the transition range is possible. The element allows changes between models in an 'optimal' way without or with very little disturbances. The constraints are considered using either the Penalty Method or the Augmented Lagrange Method. A slightly modification leads to a formulation for the transition between rigid and flexible parts of structures. Examples show the efficiency and practical applicability of the derived transition elements.

REFERENCES


